

Series lectures of phase-field model

11. Eshelby's inclusion theory

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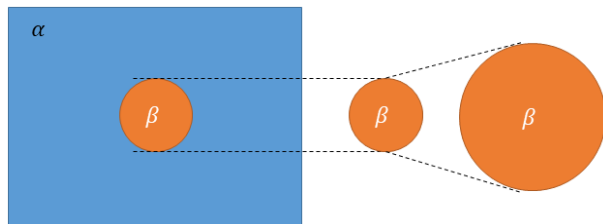


- 1 Eshelby's inclusion theory
 - Eigenstrain
 - Mechanical Equilibrium and Eshelby inclusion theory

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Effects of elasticity on Phase transformations

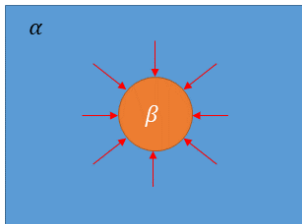
- Source of stress 1: Lattice parameter can be a function of composition: Lattice planes are continuous, no dislocation.
- Source of stress 2: Difference in lattice parameters between two phases: Eigenstrain, misfit strain



- Make β phase lattice parameter match α - coherent.
- Eigenstrain

$$\varepsilon_{ij} = \frac{a^\beta - a^\alpha}{a^\alpha} \delta_{ij}$$

where a^β is lattice parameter of β phase and a^α is lattice parameter of α phase.



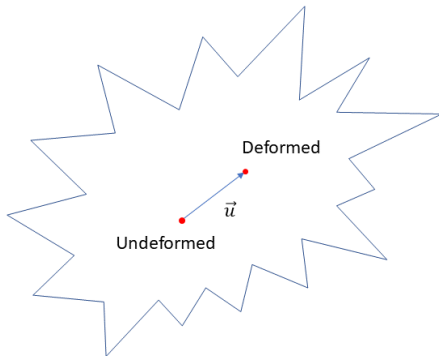
Elastic stress surrounding a coherent spherical precipitate

- Coherent lattice
- Dilatational misfit
- Linear isotropic inhomogeneous elasticity
- Inhomogeneous elastic constant of α and β can be different.
- Spherical particle

- Strain is related to the gradients in the displacement

$$\varepsilon_{ij} = \frac{1}{2}(u_{i,j} + u_{j,i}) \quad 3 \times 3 \text{ tensor}$$

$$u_{i,j} = \frac{\partial u_i}{\partial x_j}$$



- With 4th-order rank elastic constant

$$\sigma_{ij} = C_{ijkl}\varepsilon_{kl}$$

implicitly means (Einstein notation)

$$\sigma_{ij} = \sum_{k=1}^3 \sum_{l=1}^3 C_{ijkl}\varepsilon_{kl}$$

- For isotropic materials

$$\sigma_{ij} = \lambda\varepsilon_{kk}\delta_{ij} + 2\mu\varepsilon_{ij}$$

where λ is Lamé's constant and μ is the shear modulus.

- Condition for equilibrium

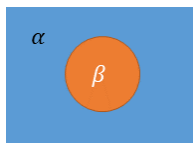
$$\nabla \cdot [\sigma] = 0 \rightarrow \sigma_{ij,j} = 0$$

- For an isotropic material

$$(\lambda + \mu)\nabla(\nabla \cdot \mathbf{u}) + \mu\nabla^2\mathbf{u} = 0$$

- In spherical coordinates, for spherical particle at $\alpha - \beta$ interface coherent

$$\mathbf{u}^\alpha(R) = \mathbf{u}^\beta(R)$$



- Force balance at interface

$$\sigma_{rr}^{\beta}(R) - \sigma_{rr}^{\alpha}(R) = -\frac{2f}{R}$$

- For spherical particle and assume the spherical symmetry

$$\mathbf{u} = u\hat{r}$$

then the force balance equation becomes

$$\frac{d^2u}{dr^2} + \frac{2}{r} \frac{du}{dr} - \frac{2u}{r^2} = 0$$

$$\frac{d}{dr} \left[\frac{1}{r^2} \frac{d(r^2u)}{dr} \right] = 0 \rightarrow \frac{d(r^2u)}{dr} = A'r^2$$

$$u = Ar + \frac{B}{r^2}$$



- When $r \rightarrow 0$, u have to be finite. Therefore, $B = 0$.
- When $r \rightarrow \infty$, $u \rightarrow 0$. Therefore, $A = 0$
- Therefore,

$$u^\beta = Ar \qquad u^\alpha = \frac{B}{r^2}$$

- According to continuum mechanics,

$$\varepsilon_{rr} = \frac{du}{dr} \qquad \varepsilon_{\theta\theta} = \varepsilon_{\phi\phi} = \frac{u}{r}$$

- Eigenstrains, misfit strains

$$\underbrace{\varepsilon_{ij}}_{\text{Total strain compatible}} = \underbrace{\varepsilon_{ij}^e}_{\text{elastic strain}} + \underbrace{\varepsilon_{ij}^o}_{\text{eigenstrain}}$$

- By Hooke's law

$$\sigma_{ij}^\beta = C_{ijkl}^\beta \varepsilon_{ij}^{\beta,e} = C_{ijkl}^\beta (\varepsilon_{ij}^\beta - \varepsilon_{ij}^o)$$

- Assuming no holes or overlap of material

$$\varepsilon_{ij} = \frac{1}{2}(u_{i,j} + u_{j,i})$$

- For isotropic materials and assume $\varepsilon_{ij}^{\circ} = \varepsilon\delta_{ij}$

$$\sigma_{ij}^{\beta} = \lambda^{\beta}\varepsilon_{kk}^{\beta}\delta_{ij} + 2\mu^{\beta}\varepsilon_{ij}^{\beta} - 3K^{\beta}\varepsilon\delta_{ij}$$

where K^{β} is bulk modulus of β phase.

$$\sigma_{ij}^{\alpha} = \lambda^{\alpha}\varepsilon_{kk}^{\alpha}\delta_{ij} + 2\mu^{\alpha}\varepsilon_{ij}^{\alpha}$$

In other notations, we have

$$\sigma^{\alpha} = \sigma_{11}^{\alpha} = \sigma_{22}^{\alpha} = \sigma_{33}^{\alpha} = (C_{11}^{\alpha} + 2C_{12}^{\alpha})\varepsilon^{\alpha} \quad (1)$$

where

$$\varepsilon^{\alpha} = \varepsilon_{11}^{\alpha} = \varepsilon_{22}^{\alpha} = \varepsilon_{33}^{\alpha}$$



- Note that

$$C_{11} = \lambda + 2\mu \quad C_{12} = \lambda$$

- Inside precipitate,

$$\begin{aligned}
 \sigma_{rr}^{\beta} &= \lambda^{\beta} [\varepsilon_{rr}^{\beta} + \varepsilon_{\theta\theta}^{\beta} + \varepsilon_{\phi\phi}^{\beta}] + 2\mu^{\beta} \varepsilon_{rr}^{\beta} - 3K^{\beta} \varepsilon \\
 &= \lambda^{\beta} \left[\frac{du^{\beta}}{dr} + \frac{2u^{\beta}}{r} \right] + 2\mu^{\beta} \frac{du^{\beta}}{dr} - 3K^{\beta} \varepsilon \\
 &= \lambda^{\beta} [A + 2A] + 2\mu^{\beta} \times A - 3K^{\beta} \varepsilon \\
 &= 3K^{\beta} (A - \varepsilon)
 \end{aligned}$$

where

$$K^{\beta} = \frac{3\lambda^{\beta} + 2\mu^{\beta}}{3}$$

- For matrix

$$\begin{aligned}
 \sigma_{rr}^{\alpha} &= \lambda^{\alpha} \left[\frac{du^{\alpha}}{dr} + \frac{2u^{\alpha}}{r} \right] + 2\mu^{\alpha} \frac{du^{\alpha}}{dr} \\
 &= \lambda^{\alpha} \left[-\frac{2B}{r^3} + \frac{2B}{r^3} \right] - \frac{4\mu^{\alpha} B}{r^3} = -\frac{4\mu^{\alpha} B}{r^3}
 \end{aligned}$$



- At interface

$$\sigma_{rr}^{\alpha}(R) = -\frac{4\mu^{\alpha}B}{R^3}$$

- Apply force balance at interface

$$\sigma_{rr}^{\beta}(R) - \sigma_{rr}^{\alpha}(R) = \frac{4\mu^{\alpha}B}{R^3} + 3K^{\beta}(A - \varepsilon) = -\frac{2f}{R}$$

- Displacement condition

$$u^{\alpha}(R) = u^{\beta}(R) \rightarrow AR = \frac{B}{R^2} \rightarrow B = AR^3$$

- We have

$$A = \frac{3K^{\beta}\varepsilon - 2f/R}{3K^{\beta} + 4\mu^{\alpha}}$$

Evaluating stress and strain

- Inside the particle

$$\varepsilon_{rr}^{\beta} = \varepsilon_{\theta\theta}^{\beta} = \varepsilon_{\phi\phi}^{\beta} = A$$

$$\sigma_{rr}^{\beta} = \sigma_{\theta\theta}^{\beta} = \sigma_{\phi\phi}^{\beta} = 3K^{\beta}(A - \varepsilon)$$

where

$$\varepsilon = \frac{1}{3} \frac{V_m^{\beta} - V_m^{\alpha}}{V_m^{\alpha}}$$

- For matrix,

$$\varepsilon_{rr}^{\alpha} = \frac{du}{dr} = -\frac{2B}{r^3} \quad \varepsilon_{\theta\theta}^{\alpha} = \varepsilon_{\phi\phi}^{\alpha} = \frac{B}{r^3}$$

$$\sigma_{rr}^{\alpha} = -\frac{4B\mu^{\alpha}}{r^3} \quad \sigma_{\theta\theta}^{\alpha} = \sigma_{\phi\phi}^{\alpha} = 2\mu^{\alpha} \left(\frac{B}{r^3} \right)$$

- Note that

$$\sigma_{rr}^{\alpha} + \sigma_{\theta\theta}^{\alpha} + \sigma_{\phi\phi}^{\alpha} = 0$$



Stress in precipitate

- Stress field and strain field are constants.
- Pure hydrostatic pressure.
- Assume no surface stress $f = 0$,

$$A = \frac{3K^\beta \varepsilon}{3K^\beta + 4\mu^\alpha} \quad \sigma_{rr}^\beta = \frac{-4\mu^\alpha \varepsilon}{3K^\beta + 4\mu^\alpha}$$

- For case $\varepsilon > 0 \rightarrow \sigma_{rr} < 0$, it means particle is under compression.
($V_m^\beta > V_m^\alpha$)
- For the case of $f \neq 0$ and $\varepsilon = 0$

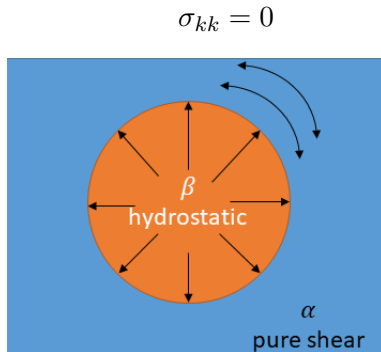
$$A = \frac{1}{3K^\beta} \left(-\frac{2f}{R} \right) \quad \sigma_{rr}^\beta = \sigma_{\theta\theta}^\beta = \sigma_{\phi\phi}^\beta = -\frac{2f}{R}$$

The pressure inside the particle

$$P^\beta = -\frac{1}{3} \sigma_{kk} = \frac{2f}{R}$$



- Pure shear stress



Stress at interface

